

Qus 1

- Ans (i) D (v) C
(ii) A (vi) C
(iii) D (vii) B
(iv) D

Qus 2

Ans The computation of population by various methods can be illustrated through table given below:—

Prediction of Population

Year	Population	Increase per decade	Incremental Increase	Percentage Increase	Decrease in Percentage Increase
1940	20,000	—	—	—	—
1950	24,500	4,500	—	22.5	—
1960	29,500	5,000	500	20.4	2.1
1970	35,200	5,700	700	19.3	1.1
Total		15,200	12,00	62.2	3.2
Average		5,070	600	20.7	1.6

Population in 1980:—

- 1> Arithmetical Progression Method = $35,200 + 5,070 = 40,270$
- 2> Geometrical Progression Method

$$= 35,200 + \frac{20.7}{100} \times 35,200 = 42,486$$
- 3> Incremental Increase Method

$$= 35,200 + 5,070 + 600 = 40,870$$
- 4> Changing Increase Rate method

$$= 35,200 + \frac{19.3 - 1.6}{100} \times 35,200 = 41,430$$

Ques
Ans

Design a rapid sand filter unit for 4.5 mld

a) Filter size

= Design discharge = 4.5 mld

with 4% as the allowance for washwater, total discharge to be treated in the filter = $\frac{4.68 \times 24}{23.5} = 4.78 \text{ mld}$

Let rate of filtration be $5,000 \text{ lits/m}^2/\text{hr.}$
 $= 5 \text{ m}^3/\text{m}^2/\text{hr.}$

Filter area required

$$= \frac{4.78 \times 10^6}{24 \times 5000} = 39.8 \text{ m}^2$$

Providing 2 No filter units, each of size $5\text{m} \times 4\text{m}$
 $\left(\frac{L}{B} = 1.25\right)$

Total area provided = $2 \times 5 \times 4 = 40 \text{ m}^2$

b) Depth of sand bed

= Assume depth of sand = 60 cm

effective size = 0.65 mm

Terminal loss of head = 2.5 m

Break-through index $B = 4 \times 10^{-4}$

$$H_s = \frac{2 \times 5 \times 0.65^3 \times 2.5}{29323 \times 4 \times 10^{-4}} = 0.58 \text{ m}$$

c) Depth of Gravel bed

= Assuming gravel size gradation of 3 mm

at top increasing to 50 mm

Provide a gravel depth of 45 cm

d) Underdrains

Total area perforations

= 0.2% of total filter area.

$$= \frac{1}{500} \times 5 \times 4 \text{ [for each unit]}$$

$$= 0.04 \text{ m}^2$$

Laterals may be assumed to have a spacing of 15cm (Max^m 30cm)

Total no. of laterals

$$= \frac{5 \times 100}{15} = 33.3 \text{ say } 34$$

Length of lateral = $\frac{1}{2}$ width of filter - $\frac{1}{2}$ dia of a manifold

$$= 4 \times 100 / 2 - 45 / 2 = 177.5 \text{ cm}$$

$$= 1.775 \text{ m}$$

Let n be total no of perforations of 13mm dia

$$\begin{aligned} \text{Total area of perforations} &= 0.04 \text{ m}^2 \\ &= 400 \text{ cm}^2 \end{aligned}$$

$$n \times \frac{\pi}{4} \times 13^2 = 400$$

$$n \times 1.33 = 400$$

$$n = 300$$

No of perforations per lateral

$$= \frac{300}{68} = 4.47$$

Area of perforations per lateral

$$= 4.47 \times 1.33 = 5.94 \text{ cm}^2$$

Now area of lateral = 2 x area of perforation per lateral

$$= 2 \times 5.95 = 11.88 \text{ cm}^2$$

$$\text{Dia of lateral} = 3.89 \text{ cm}$$

$$\frac{\text{Length of lateral}}{\text{Dia of lateral}} = \frac{177.5}{3.89}$$

$$= 45.6 < 60$$

Let rate of washing be 45 cm/m^{hr}

Then wash water discharge

$$= \frac{45 \times 5 \times 4}{100 \times 60} = 0.15 \text{ cumecs}$$

$$\text{Velocity of flow in lateral} = \frac{0.15 \times 10,000}{68 \times 11.88}$$

$$= 1.86 \text{ m/sec}$$